CHAPTER 4 STRESS AND STRAIN

EXERCISE 24, Page 95

1. If a solid stone is dropped into the sea and comes to rest at a depth of 5000 m below the surface of the sea, what will be the stress in the stone? Take the density of seawater = 1020 kg/m^3 and $g = 9.81 \text{ m/s}^2$.

Stress in stone, $\sigma = \rho gh = 1020 \times 9.81 \times -5000$

= - 50 MN/m²

2. A solid bar of length 1 m consists of three shorter sections firmly joined together. Assuming the following apply, determine the change in length of the bar when it is subjected to an axial pull of 50 kN. Assume Young's modulus, $E = 2 \times 10^{11} \text{ N/m}^2$.

Section	Length (m)	Diameter (mm)
1	0.2	15
2	0.3	20
3	0.5	30

$$A_{I} = \frac{\pi \times (15 \times 10^{-3})^{2}}{4} = 1.767 \times 10^{-4} \text{ m}^{2}, \quad A_{2} = \frac{\pi \times (20 \times 10^{-3})^{2}}{4} = 3.142 \times 10^{-4} \text{ m}^{2}$$

and $A_3 = \frac{\pi \times (30 \times 10^{-3})^2}{4} = 7.069 \times 10^{-4} \text{ m}^2$

$$\sigma_1 = \frac{F}{A_1} = \frac{50 \times 10^3}{1.767 \times 10^{-4}} = 283 \text{ MN/m}^2, \quad \sigma_2 = \frac{F}{A_2} = \frac{50 \times 10^3}{3.142 \times 10^{-4}} = 159.1 \text{ MN/m}^2$$

and $\sigma_3 = \frac{F}{A_3} = \frac{50 \times 10^3}{7.069 \times 10^{-4}} = 70.73 \text{ MN/m}^2$

$$\delta = \frac{283 \times 10^6 \times \ 0.2 \ + \ 159.1 \times 10^6 \times \ 0.3 \ + \ 70.7 \times 10^6 \times \ 0.5}{2 \times 10^{11}} \ 1$$

3. If the bar of Problem 2 were made from three different materials with the following elastic moduli, determine the change in length of the bar:

Section	$E(N/m^2)$
1	2×10^{11}
2	7×10^{10}
3	1×10^{11}

$$\delta = \frac{283 \times 10^6 \times 0.2}{2 \times 10^{11}} + \frac{159.1 \times 10^6 \times 0.3}{7 \times 10^{10}} + \frac{70.7 \times 10^6 \times 0.5}{1 \times 10^{11}}$$

i.e. $\delta = 1.318 \times 10^{-3} \text{ m} = 1.318 \text{ mm}$

4. A circular-section solid bar of linear taper is subjected to an axial pull of 0.1 MN, as show. If $E = 2 \times 10^{11} \text{ N}/\text{m}^2$, by how much will the bar extend?



$$u = \frac{Wl}{A_1 E} \left(\frac{d_1}{d_2}\right)$$
$$A_1 = \frac{\pi \times (1 \times 10^{-2})^2}{4} = 7.854 \times 10^{-5} \text{ m}^2$$
$$u = \frac{0.1 \times 10^6 \times 1}{7.854 \times 10^{-5} \times 2 \times 10^{11}} \left(\frac{1}{8}\right)$$

Hence,

$$=7.958 \times 10^{-4} \,\mathrm{m} = u_1 \tag{1}$$

i.e. bar extension, u = 0.796 mm

Proof of formula for this problem

$$d_1 = lcm.$$

$$d_2 = 8cm.$$

$$d_2 = 8cm.$$

$$d_3 = 100$$

At x,
$$d = 1 \times 10^{-2} + 7 \times 10^{-2} x$$

i.e.
$$d = 1 \times 10^{-2} (1 + 7x)$$

At x,
$$\sigma = \frac{W}{\pi d^2 / 4} = \frac{W \times 4}{\pi \left[1 \times 10^{-2} (1 + 7x)\right]^2}$$

i.e.

$$\sigma = \frac{12732W}{\left(1+7x\right)^2}$$

At x,
$$\varepsilon = \frac{12732W}{(1+7x)^2 \times 2 \times 10^{11}}$$

$$=\frac{6.366\times10^{-8}W}{(1+7x)^2}$$

and

$$\delta u = \frac{6.366 \times 10^{-8} \, W \, \delta x}{\left(1 + 7x\right)^2}$$

Thus,

$$\mathbf{u} = 6.366 \times 10^{-8} W \int_0^1 \frac{1}{\left(1 + 7x\right)^2} \, dx$$

$$= 6.366 \times 10^{-8} \times 0.1 \times 10^{6} \left[\frac{-1}{7(1+7x)} \right]_{0}^{1}$$
$$= 6.366 \times 10^{-8} \times 0.1 \times 10^{6} \left[\left(\frac{-1}{56} \right) - \left(\frac{-1}{7} \right) \right]_{0}^{1}$$

i.e.

 $u = 7.96 \times 10^{-4} \text{ m} = 0.796 \text{ mm}$

5. If a solid stone is dropped into the sea and comes to rest at a depth of 11000 m below the surface of the sea, what will be the stress in the stone? Assume that density of sea water = 1020 kg/m^3 and $g = 9.81 \text{ m/s}^2$.

Stress in stone, $\sigma = \rho gh = 1020 \times 9.81 \times -11000$

=-110 MPa

6. A solid bar of length 0.7 m consists of three shorter sections firmly joined together. Assuming the following apply, determine the change in length of the bar when it is subjected to an axial pull of 30 kN. Assume that Young's modulus, $E = 2 \times 10^{11} \text{ N/m}^2$.

Section	Length (m)	Diameter (mm)
1	0.1	10
2	0.2	15
3	0.4	20

$$A_{I} = \frac{\pi \times (10 \times 10^{-3})^{2}}{4} = 7.854 \times 10^{-5} \,\mathrm{m}^{2} \,, \quad A_{2} = \frac{\pi \times (15 \times 10^{-3})^{2}}{4} = 1.767 \times 10^{-4} \,\mathrm{m}^{2-2}$$

and $A_3 = \frac{\pi \times (20 \times 10^{-3})^2}{4} = 3.142 \times 10^{-4} \text{ m}^2$

$$\sigma_1 = \frac{F}{A_1} = \frac{30 \times 10^3}{7.854 \times 10^{-5}} = 382 \text{ MN/m}^2, \quad \sigma_2 = \frac{F}{A_2} = \frac{30 \times 10^3}{1.767 \times 10^{-4}} = 169.8 \text{ MN/m}^2$$

and $\sigma_3 = \frac{F}{A_3} = \frac{30 \times 10^3}{3.142 \times 10^{-4}} = 95.48 \text{ MN/m}^2$

$$\delta = \frac{382 \times 10^6 \times \ 0.1 \ + \ 169.8 \times 10^6 \times \ 0.2 \ + \ 95.48 \times 10^6 \times \ 0.4}{2 \times 10^{11}} 2$$

i.e. the change in length of bar, $\delta = 5.5176 \times 10^{-4} \text{ m} = 0.552 \text{ mm}$

7. If the bar of question 6 were made from three different materials with the following elastic moduli, determine the change in length of the bar.

Section	$E(N/m^2)$
1	2×10^{11}
2	7×10^{10}
3	1×10 ¹¹

$$\delta = \frac{382 \times 10^6 \times 0.1}{2 \times 10^{11}} + \frac{169.8 \times 10^6 \times 0.2}{7 \times 10^{10}} + \frac{95.48 \times 10^6 \times 0.4}{1 \times 10^{11}}$$

i.e. $\delta = 1.058 \times 10^{-3} \text{ m} = 1.058 \text{ mm}$

8. A circular-section solid bar of linear taper is subjected to an axial pull of 0.2 MN, as shown. If E = 2×10^{11} N/m², by how much will the bar expand?

From equation (1) of Problem 4,

$$u = \frac{Wl}{A_1 E} \left(\frac{d_1}{d_2}\right)$$
$$A_1 = \frac{\pi \times (1 \times 10^{-2})^2}{4} = 7.854 \times 10^{-5} \text{ m}^2$$
$$u = \frac{0.2 \times 10^6 \times 1}{7.854 \times 10^{-5} \times 2 \times 10^{11}} \left(\frac{1}{8}\right)$$

Hence,

$$= 1.592 \times 10^{-3} \,\mathrm{m}$$

i.e. bar extension, u = 1.592 mm

1. If the bar of question 4 in Practise Exercise 24, page 95, were prevented from moving axially by two rigid walls and subjected to a temperature rise of 10°C, what would be the maximum stress in the bar? Assume the 0.1 MN load is not acting. Assume that coefficient of linear expansion, $\alpha = 15 \times 10^{-6} / ^{\circ}C$.

Free expansion = $\alpha lT = 15 \times 10^{-6} \times 1 \times 10 = 1.5 \times 10^{-4} \text{ m}$

From equation (1) in the solution to Problem 4, Exercise 24,

$$u=1.5\times10^{-4}=u_1\times\frac{W_T}{W}$$

so that

$$1.5 \times 10^{-4} = 7.96 \times 10^{-4} \times \left(\frac{W_T}{0.1 \times 10^6}\right)$$

from which, $W_T = 18844 N$

At the smaller end,

maximum stress in bar,
$$\sigma = \frac{-18844}{7.854 \times 10^{-5}} = -240 \text{ MN/m}^2$$

2. If the bar in Problem 4, Exercise 24, were prevented from moving axially by two rigid walls and subjected to a temperature rise of 10°C, what would be the maximum stress in the bar? Assume the 0.2 MN load is not acting, and $\alpha = 15 \times 10^{-6} / °C$.

Free expansion = $\alpha lT = 15 \times 10^{-6} \times 1 \times 10 = 1.5 \times 10^{-4} \text{ m}$

From equation (1) in the solution to Problem 4, Exercise 24,

$$u = 1.5 \times 10^{-4} = u_1 \times \frac{W_T}{W}$$

so that

 $1.5 \times 10^{-4} = 7.96 \times 10^{-4} \times \left(\frac{W_T}{0.2 \times 10^6}\right)$

from which, $W_T = 37688 N$

At the smaller end,

maximum stress in bar, $\sigma = \frac{-37688}{7.854 \times 10^{-5}} = -$ 480 MN/m²

1. An electrical cable consists of a copper core surrounded co-axially by a steel sheath, so that the two can be assumed to act as a compound bar. If the cable hangs down a vertical mineshaft, determine the maximum permissible length of the cable, assuming the following apply: $A_c = 1 \times 10^{-4} \text{ m}^2 = \text{sectional area of copper}, E_c = 1 \times 10^{11} \text{ N/m}^2 = \text{elastic modulus of copper},$ $\rho_c = 8960 \text{ kg/m}^3 = \text{density of copper, maximum permissible stress in copper} = 30 \text{ MN/m}^2,$ $A_s = 0.2 \times 10^{-4} \text{ m}^2 = \text{sectional area of steel}, E_s = 2 \times 10^{11} \text{ N/m}^2 = \text{elastic modulus of steel},$ $\rho_s = 7860 \text{ kg/m}^3 = \text{density of steel}, \text{ maximum permissible stress in steel} = 100 \text{ MN/m}^2$ $g = 9.81 \text{ m/s}^2$



$$W = (\rho_c A_c + \rho_s A_s) g l$$

= (0.896 + 0.1572) × 9.81 l
W = 10.332 l

i.e.

Compatibility

$$\delta_c = \delta_s$$

$$l imes \varepsilon_c = l imes \varepsilon_s$$

$$\frac{O_c}{E_c} = \frac{O_s}{E_s}$$

 $\sigma_c = 0.5\sigma_s$

from which,

$$\overline{c}_c = \frac{E_c}{E} \sigma_s = \frac{1 \times 10^{11}}{2 \times 10^{11}} \sigma_s$$

 σ_c is the design criterion, i.e. $\sigma_c = 30$ MN/m²

and

$$\sigma_{\rm s} = 60$$
 MN/m²

Equilibrium

 $W = \sigma_c A_c + \sigma_s A_s$

or

$$10.332 \ l = (30 \times 10^6 \times 1 \times 10^{-4} + 60 \times 10^6 \times 0.2 \times 10^{-4})$$

$$l = \frac{(30 \times 10^{6} \times 1 \times 10^{-4} + 60 \times 10^{6} \times 0.2 \times 10^{-4})}{10.332}$$

i.e. the maximum permissible length of the cable, l = 406.5 m

2. How much will the cable of question 1 stretch, owing to self-weight?

Average stress in copper = 15 MN/m^2

Therefore, average strain = $\frac{15 \times 10^6}{1 \times 10^{11}} = 1.5 \times 10^{-4}$

Hence, **cable stretch**, $\delta = 1.5 \times 10^{-4} \times 406.5 = 0.06098$ m

= **61 mm**

Check: σ_{AV} (steel) = 30 MN/m²

 ε_{AV} (steel) = 1.5×10^{-4}

and

 $\delta = 1.5 \times 10^{-4} \times 406.5 = 0.061 \text{ m} = 61 \text{ mm}$

3. If a weight of 100kN were lowered into the sea, via a steel cable of cross-sectional area $8 \times 10^{-4} \text{ m}^2$, what would be the maximum permissible depth that the weight could be lowered if the following apply?

Density of steel = 7860 kg/m^3 , density of sea water = 1020 kg/m^3 ,

maximum permissible stress in steel = 200MN/m², g = 9.81 m/s²

Any buoyancy acting on the weight itself may be neglected.

 $W = (100,000 + (7860 - 1020) \times 8 \times 10^{-4} lg)$

i.e. W = 100,000 + 53.68 l (1)

However,

$$W = \sigma A = 200 \times 10^6 \times 8 \times 10^{-4}$$

= 160,000 N (2)

Equating equations (1) and (2) gives:

$$160,000 = 100,000 + 53.68 l$$

i.e. the maximum permissible depth, l = 60000/53.68 = 1118 m

4. A weightless rigid horizontal beam is supported by two vertical wires, as shown. If the following apply, determine the position from the left that a weight W can be suspended, so that the bar will remain horizontal when the wires stretch.

Left wire: cross-sectional area = 2A, elastic modulus = E, length = 2l

Right wire: cross-sectional area = A, elastic modulus = 3E, length = l



$$\delta_1 = \delta_2$$

$$2 l \varepsilon_1 = l \varepsilon_2$$

$$2 \frac{\sigma_1}{E_1} = \frac{\sigma_2}{E_2} 3$$

$$\sigma_1 = \frac{\sigma_2 E_1}{2E_2} 4$$

(1)

or

i.e.

Taking moments about the right wire gives:

$$F_1 l = W(l-x)$$
$$F_1 = \frac{W(l-x)}{l} 5$$

c	$5_1 \mathbf{A}_1 = \frac{W(l-x)}{l} 6$	
	$\sigma_1 = \frac{W(l-x)}{A_1 l} 7$	(2)
	$F_1 + F_2 = W$	
	$\sigma_1 A_1 + \sigma_2 A_2 = W$	
	$\frac{W(l-x)}{A_1 l} \times A_1 + \sigma_1 \times \frac{2E_2}{E_1} \times A_2 = W \text{from equations (1) and (2)}$	
i.e.	$\frac{W(l-x)}{l} + \frac{W(l-x)}{A_{l}l} \times \frac{2 \times 3E}{E} \times 0.5 \times A_{l} = W$	
i.e.	(l-x) + 3(l-x) = l	
i.e.	4l - 4x = l	
i.e.	4x = 3l	
and	x = 0.75 l	

5. An electrical cable consists of a copper core surrounded co-axially by a steel sheath, so that the two can be assumed top act as a compound bar. If the cable hangs down a vertical mineshaft, determine the maximum permissible length of the cable, assuming the following apply:

$$A_c = 2 \times 10^{-4} \text{ m}^2 = \text{sectional area of copper}, E_c = 1 \times 10^{11} \text{ N/m}^2 = \text{elastic modulus of copper},$$

 $\rho_c = 8960 \text{ kg/m}^3 = \text{density of copper}, \text{maximum permissible stress in copper} = 30 \text{ MN/m}^2,$
 $A_s = 0.1 \times 10^{-4} \text{ m}^2 = \text{sectional area of steel}, E_s = 2 \times 10^{11} \text{ N/m}^2 = \text{elastic modulus of steel},$
 $\rho_s = 7860 \text{ kg/m}^3 = \text{density of steel}, \text{maximum permissible stress in steel} = 100 \text{ MN/m}^2,$
 $g = 9.81 \text{ m/s}^2$

 $W = (\rho_c A_c + \rho_s A_s) g l$ = (1.792 + 0.0786) × 9.81 l W = 18.351 l

i.e.

Compatibility

 $\delta_c = \delta_s$ $l \times \varepsilon_c = l \times \varepsilon_s$ $\frac{\sigma_c}{E_c} = \frac{\sigma_s}{E_s}$

from which,

$$\sigma_c = \frac{E_c}{E_s} \sigma_s = \frac{1 \times 10^{11}}{2 \times 10^{11}} \sigma_s$$

i.e.

and

$$\sigma_c = 0.5\sigma_s$$

 σ_c is the design criterion, i.e. $\sigma_c = 30$ MN/m²

 $\sigma_{\rm s} = 60$ MN/m²

Equilibrium

or

$$18.351 \ l = (30 \times 10^6 \times 2 \times 10^{-4} + 60 \times 10^6 \times 0.1 \times 10^{-4})$$

i.e.
$$l = \frac{(30 \times 10^6 \times 2 \times 10^{-4} + 60 \times 10^6 \times 0.1 \times 10^{-4})}{18.351}$$

 $W = \sigma_c A_c + \sigma_s A_s$

i.e. the maximum permissible length of the cable, l = 359.7 m

6. How much will the cable of question 5 stretch, owing to self-weight?

Average stress in copper = 15 MN/m^2

Therefore, average strain = $\frac{15 \times 10^6}{1 \times 10^{11}} = 1.5 \times 10^{-4}$

Hence, **cable stretch**, $\delta = 1.5 \times 10^{-4} \times 359.7 = 0.0539$ m

Check: σ_{AV} (steel) = 30 MN/m²

 $\varepsilon_{\rm AV}$ (steel) = 1.5×10^{-4}

and $\delta = 1.5 \times 10^{-4} \times 359.7 = 0.0539 \text{ m} = 53.9 \text{ mm}$

7. If a weight of 95 kN were lowered into the sea via a steel cable of cross-sectional area 8×10^{-4} m² what would be the maximum permissible depth that the weight could be lowered if the following apply?

Density of steel = 7860 kg/m³ Density of sea water = 1020 kg/m³ Maximum permissible stress in steel = $200 \text{ MN} / \text{m}^2$ g = 9.81 m/s²

Any buoyancy acting on the weight itself may be neglected.

 $W = (95000 + (7860 - 1020) \times 8 \times 10^{-4} lg)$

i.e.

 $W = 95000 + 53.68 \, l \tag{1}$

However,

$$W=\sigma A=200\times10^{6}\times8\times10^{-4}$$

$$= 160,000 \text{ N}$$
 (2)

Equating equations (1) and (2) gives:

160,000 = 95000 + 53.68 l

i.e. the maximum permissible depth, l = 65000/53.68 = 1211 m

8. A weightless rigid horizontal beam is supported by two vertical wires, as shown. If the following apply, determine the position from the left that a weight W can suspended, so that the bar will remain horizontal when the wires stretch.

Left wire: cross-sectional area = 3A, elastic modulus = E, length = 2l

Right wire: cross-sectional area = 1.5A, elastic modulus = 3E, length = l



 $\delta_1 = \delta_2$

or

$$2 l \varepsilon_{1} = l \varepsilon_{2}$$

$$2 \frac{\sigma_{1}}{E_{1}} = \frac{\sigma_{2}}{E_{2}} 8$$

$$\sigma_{1} = \frac{\sigma_{2} E_{1}}{2E_{2}} 9$$
(1)

Taking moments about the right wire gives:

$$F_{1} l = W(1-x)$$

$$F_{1} = \frac{W(l-x)}{l} 10$$

$$\sigma_{1} A_{1} = \frac{W(l-x)}{l} 11$$

$$\sigma_{1} = \frac{W(l-x)}{A_{l}l} 12$$
(2)

Resolving vertically gives:

i.e.

i.e.

or

i.e.

i.e.

i.e.

i.e.

and

$$F_{1} + F_{2} = W$$

$$\sigma_{1}A_{1} + \sigma_{2}A_{2} = W$$

$$\frac{W(l-x)}{A_{1}l} \times A_{1} + \sigma_{1} \times \frac{2E_{2}}{E_{1}} \times A_{2} = W \quad \text{from equations (1) and (2)}$$

$$\frac{W(l-x)}{l} + \frac{W(l-x)}{A_{1}l} \times \frac{2 \times 3E}{E} \times 0.5 \times A_{1} = W$$

$$\frac{(l-x)}{l} + \frac{(l-x)}{0.5l} \times 2 \times 3 \times 0.5 \times 0.5 = 1$$

$$\frac{(l-x)}{l} + \frac{(l-x)}{0.5l} \times 2 \times 3 \times 0.5 \times 0.5 = 1$$

$$(l-x) + 3(l-x) = l$$

$$4l - 4x = l$$

$$4x = 4l - l = 3l$$

$$x = 0.75 l$$